

# **Modeling Interfaces Involving Multiple Engineered Features**

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# Thesis:

- **When one examines multiple subsystems in disposal facilities, interactions can provide surprising results. These insights should be reflected in design, but generally are not.**
- **Lower cost w/better performance is available now, better design is the low hanging fruit.**
- **Intuition and compartmentalized knowledge have served as poor guides.**

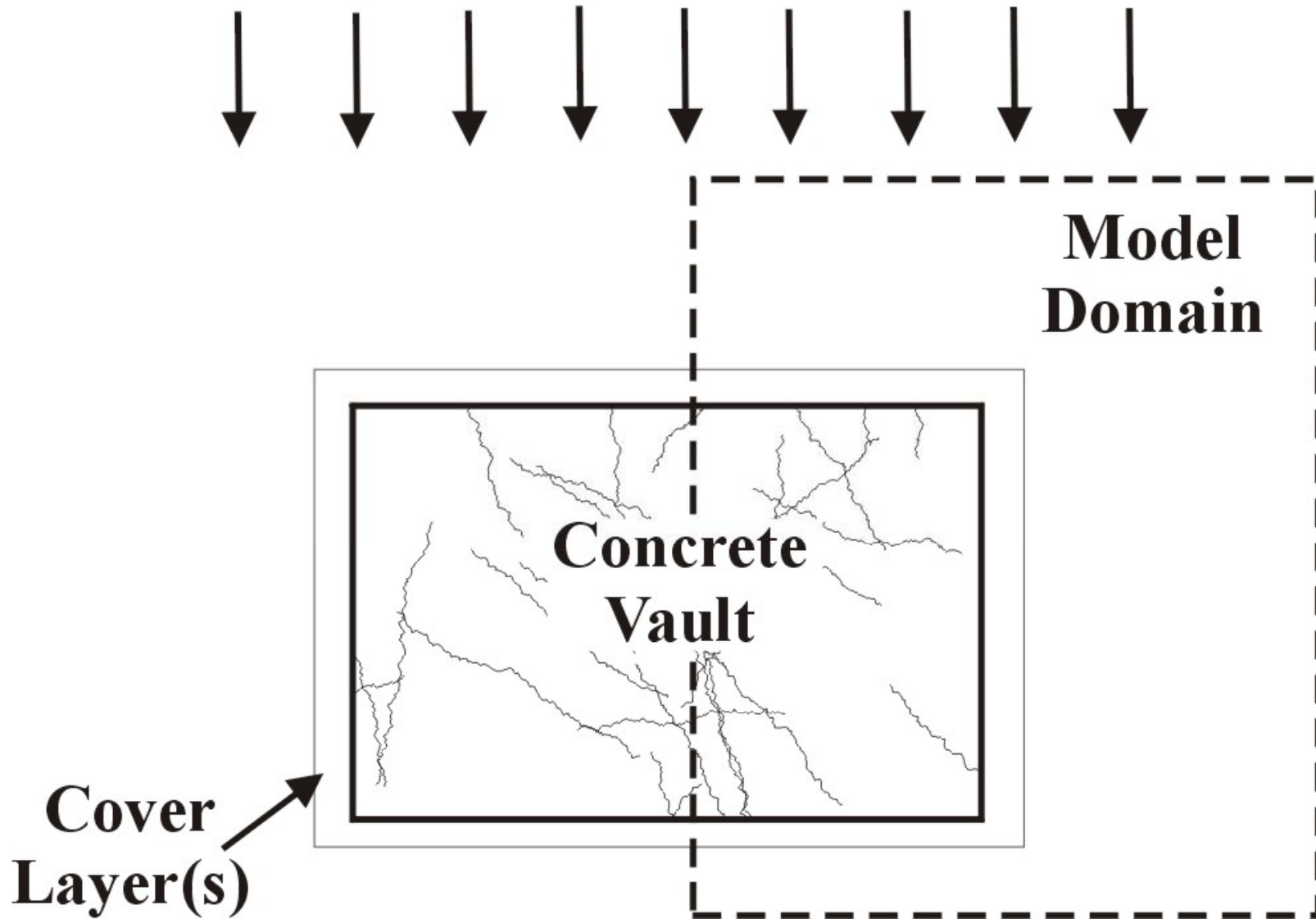
# Examples:

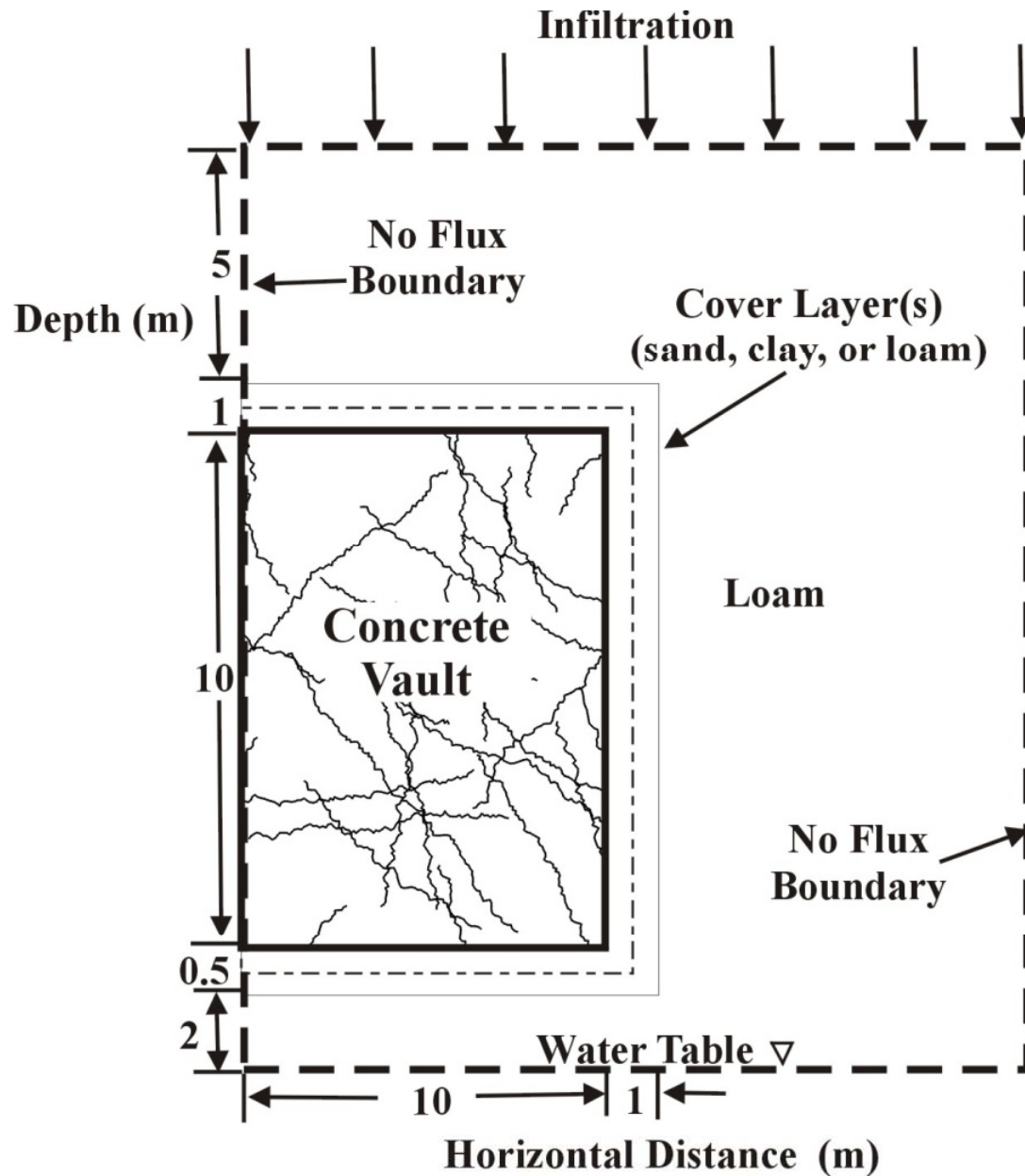
- **Scale effects on percolation**
- **Scale effects on mixing**
- **Hydraulic gradient effects**
- **Slowing barrier degradation**

# Scale Effects on Percolation

- **Below ground rectangular vault assumed**
- **Modify roof slope, size, soil type around vault, leakage through cover**
- **Cover included implicitly**
- **Estimate water flowing through vault ( $\text{cm}^3/\text{cm}^2/\text{year}$ )**
- **Rob Rice dissertation:**
  - **Design Factors Affecting the Flow of Water through Below-Ground Concrete Vaults, J. Envir. Engrg. Volume 132, Issue 10, pp. 1346-1354 (October 2006)**

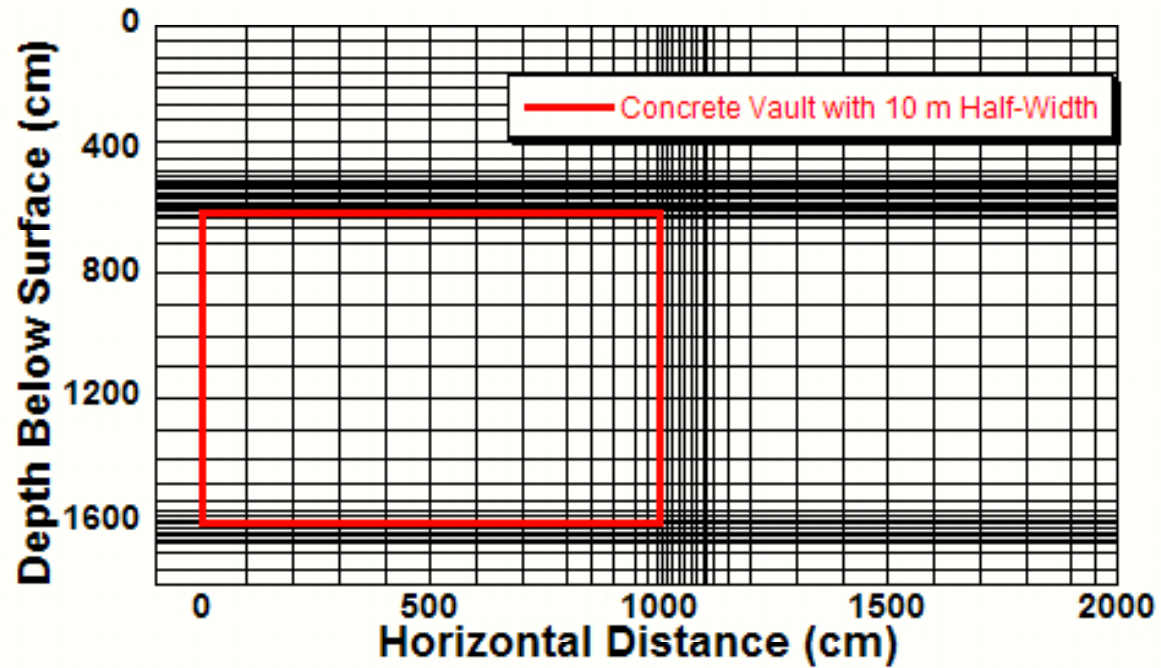
# Infiltration

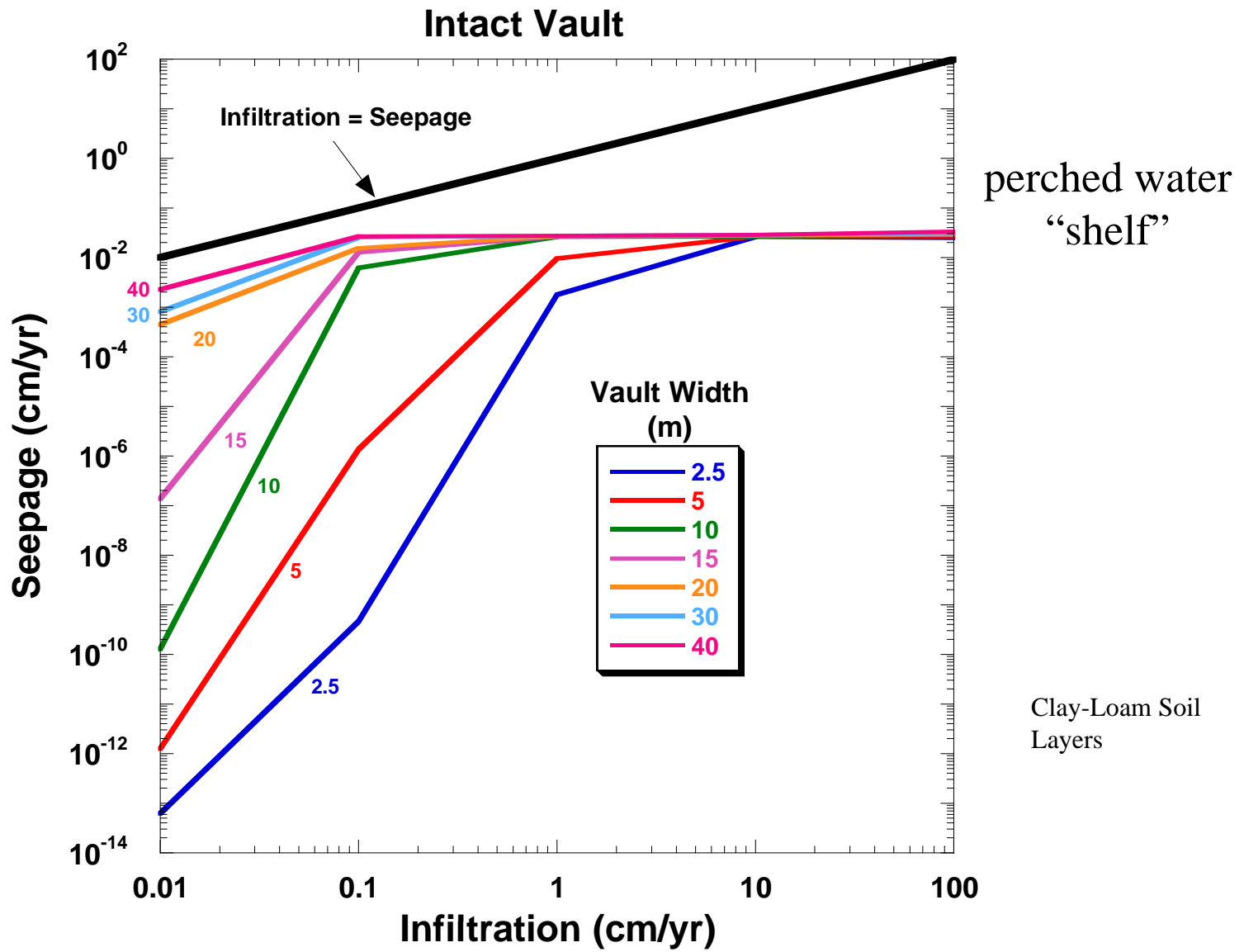




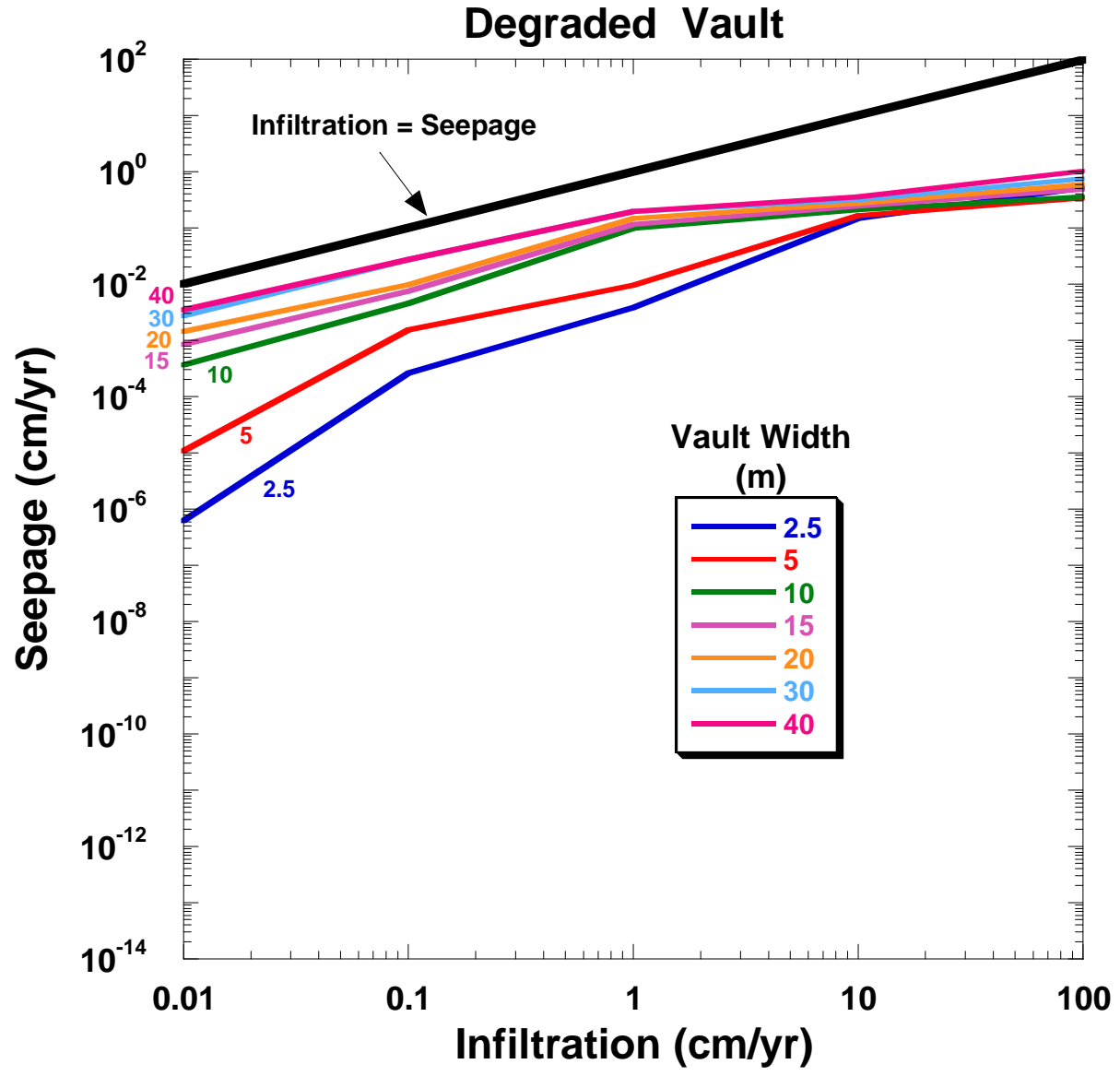
Design Factors  
Affecting the Flow of  
Water through  
Below-Ground  
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J. Envir. Engrg.  
Volume 132, Issue  
10, pp. 1346-1354  
(October 2006)

# Gridding





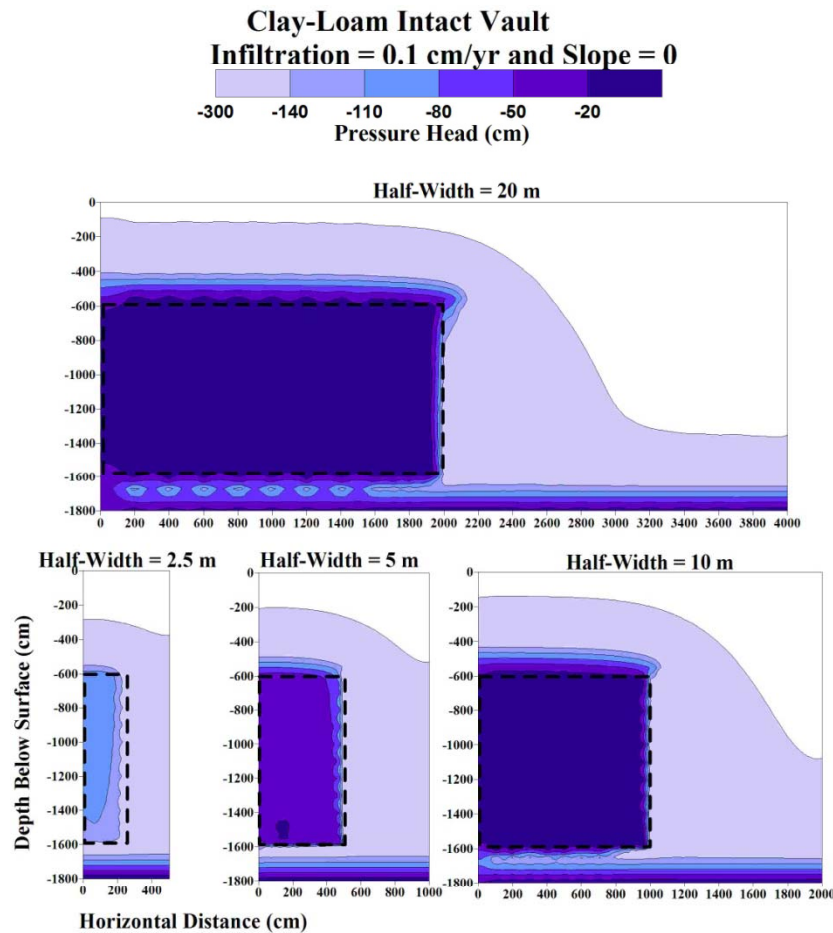




infiltration  
is  
the implicit  
leakage  
through  
cover

Clay - Loam

# Perched Water is Why

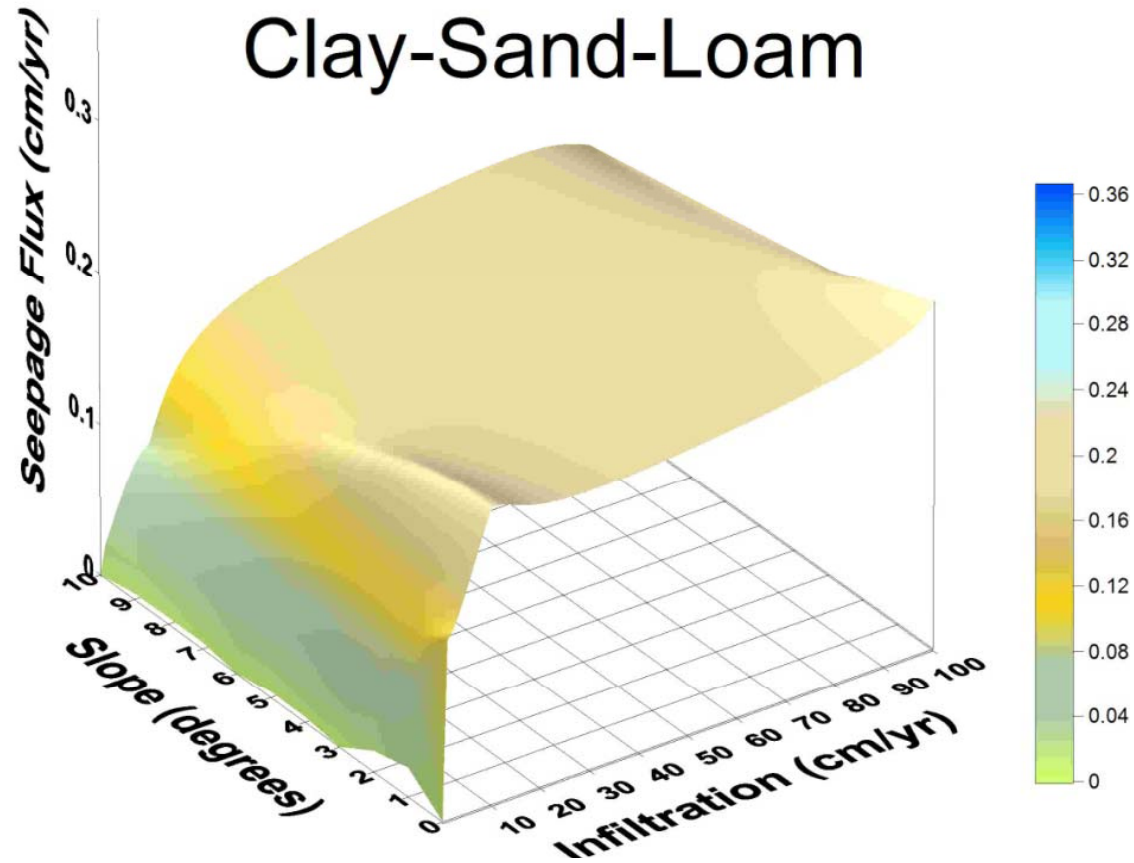


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# What happens

- **Lateral diversion of water around a cover is scale dependent**
- **Water perches over top of large vaults even at low infiltration rates**
- **Once perched water forms infiltration rate through cover becomes unimportant**
- **In general, smaller, modular vaults with individual covers perform best**
- **Modular also allows nearby infiltration of mixing water**

# Clay-Sand-Loam



Perched water shelf where seepage independent of cover leakage (infiltration)

Slope not very important

Drainage layer (sand) helps, but only a little

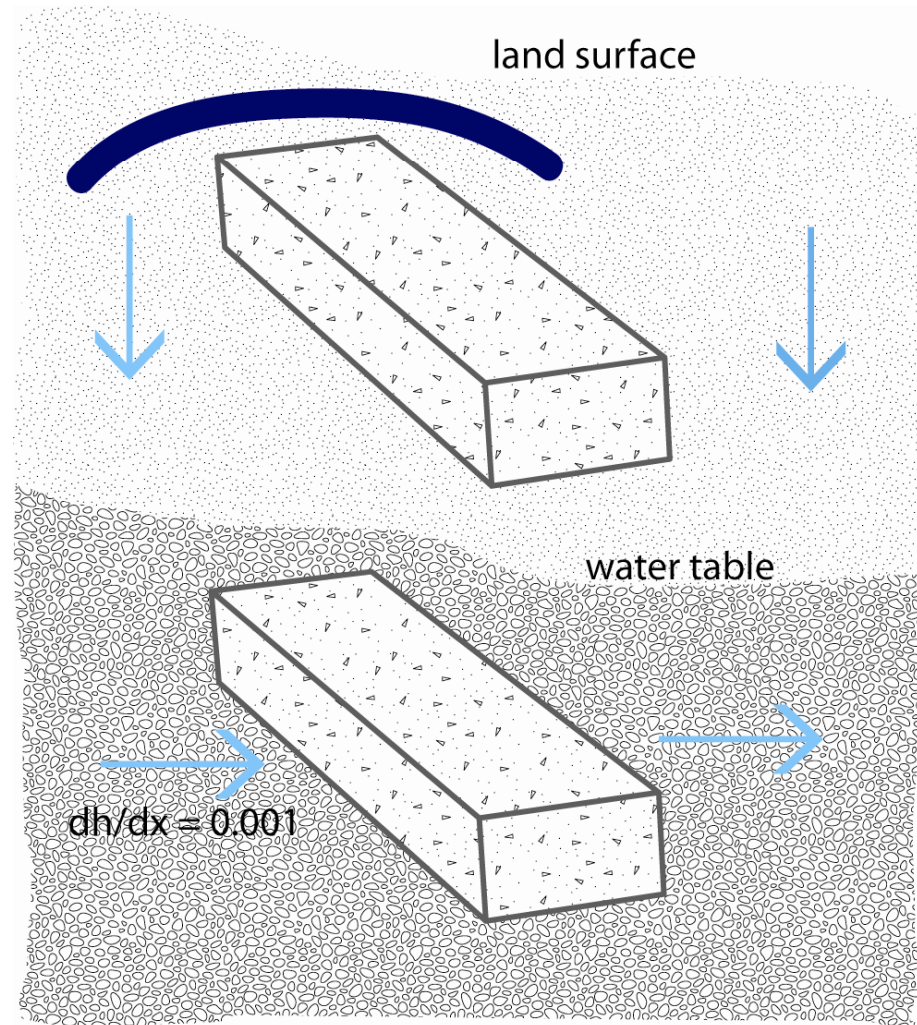
# Percolation Study Conclusions

- **Clay layers placed adjacent to the concrete lower water flow through the vault, slow degradation, and enhance hydraulic performance.**
- **Smaller vault sizes perform better.**
- **Roof slope has a relatively small influence on hydraulic performance.**
- **Covers are generally ineffective in controlling seepage**

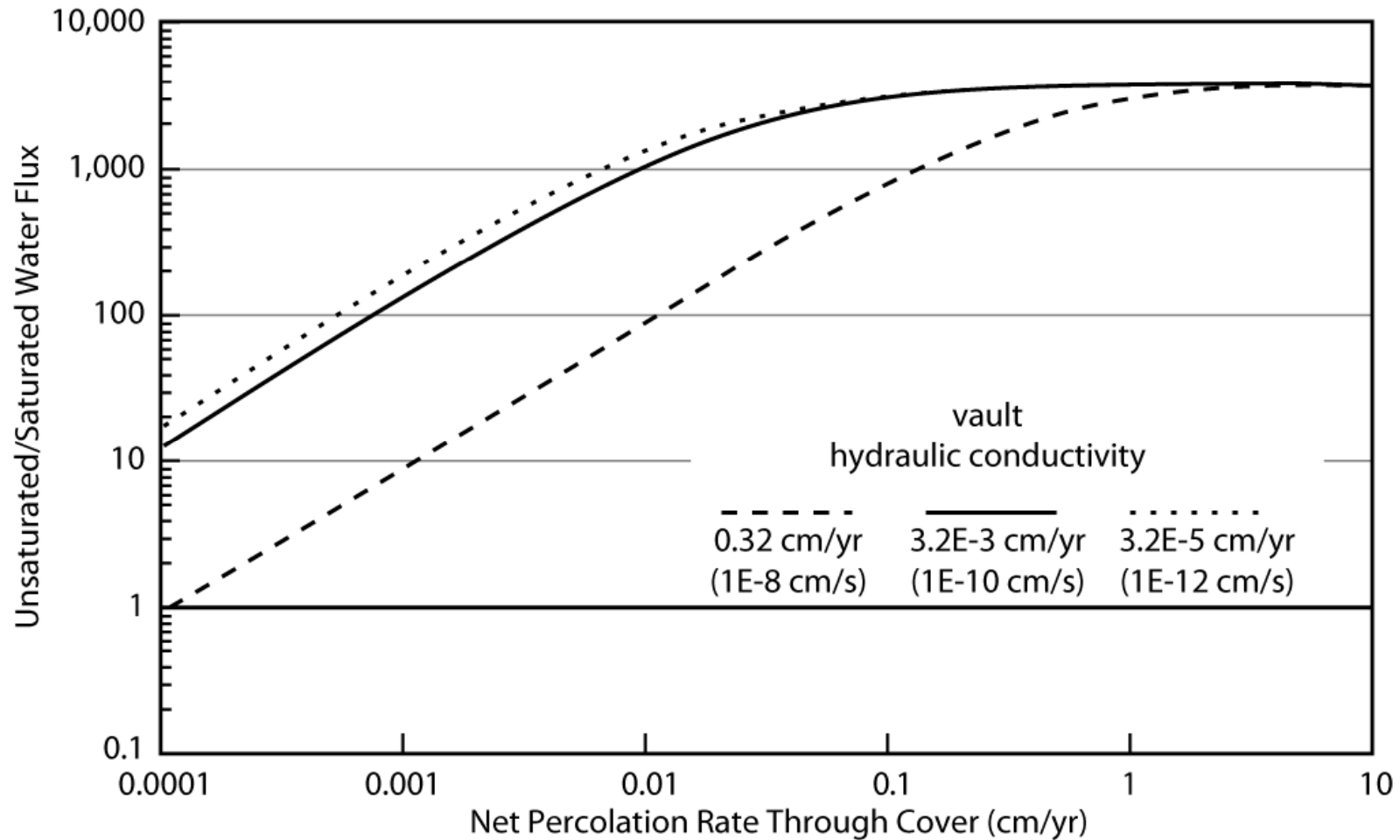
# Don't put waste below the water table!

- This is a widely held hypothesis, clearly obvious to most analysts.
- Let's do a simple numerical experiment to test the hypothesis and show how important it is.

# Numerical Test



# Turns out the obvious is wrong

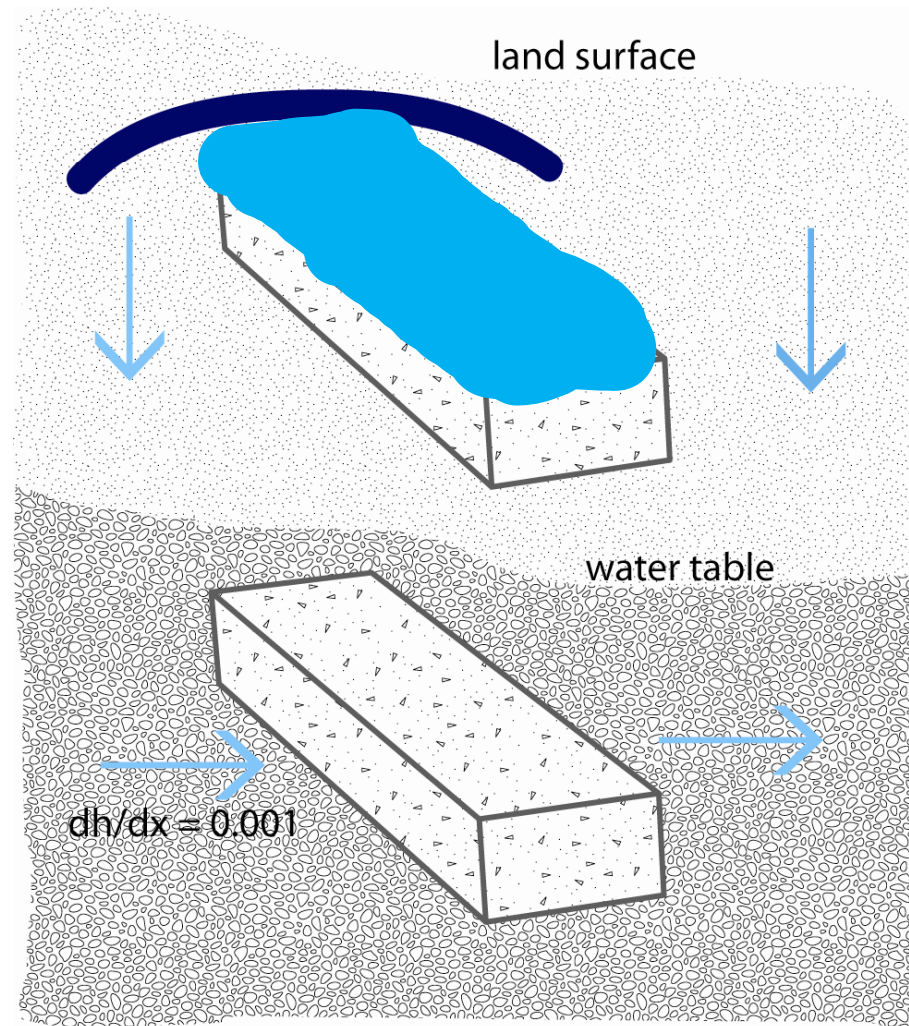




# Why Saturated Sites Work Better (Hydraulically)

- Perched water gives a unit gradient in unsaturated zone
- Typical groundwater has a low gradient (e.g.,  $1/0.001 = 1000$ )
- Top versus side of vault exposed to flow
- Unsaturated zone locations are easier to construct however

# Why



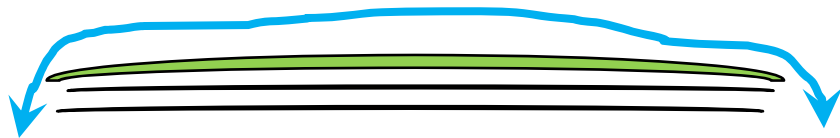
perched water  
gives  $dh/dx$   
 $\sim 1$   
fine pores in  
cementitious  
materials mean  
essentially  
saturated flow at  
both locations

relation of vault to  
flow direction also  
decreases  
performance of  
unsaturated location

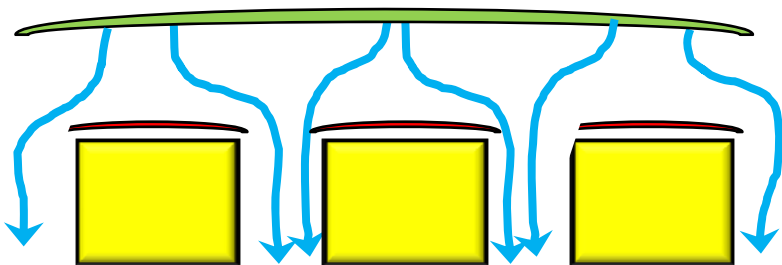
# Mixing – Peak Dose is Risk Driver

- **For long lived contaminants, peak dose**
  - **~ (release rate)/(mixing flow).**
- **Peak dose should be controlled by management of both release and mixing**
- **Minimize spikes in release, maximize mixing**
- **Remember D. Esh slide of rain giving infiltration peaks**

# Mixing



- Consider two different cover options:
  - a) large cover over entire facility or
  - b) smaller modular covers and smaller vaults

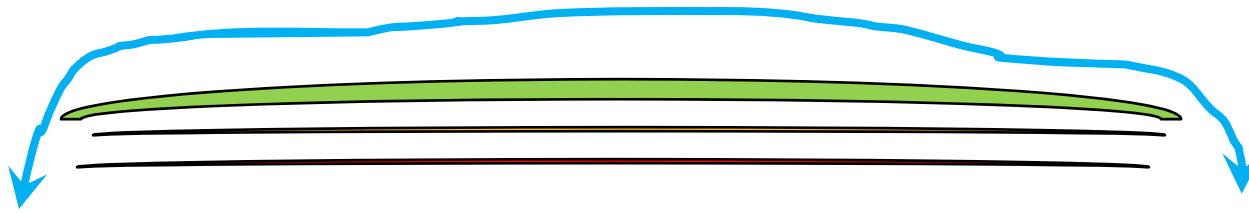


# Lowering Peak Dose

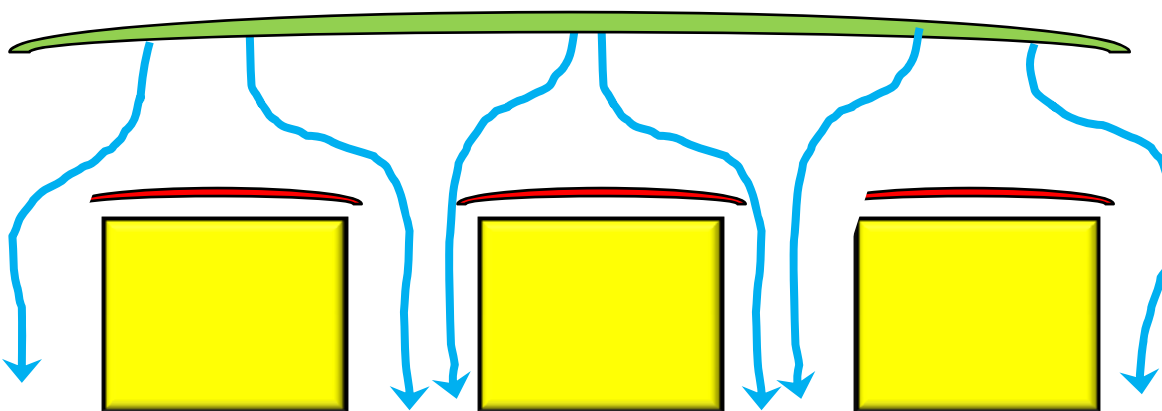
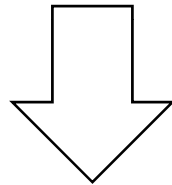
- **Smaller vaults with clay against the vault will perform better and more reliably than the typical cover – (lower release)**
- **Mixing of leachate with diverted water takes place when vault size < (distance to boundary)/10**
- **Buried (clay over structure) covers degrade more slowly – further from the surface**
- **Combination of plastic and brittle materials naturally resists subsidence and seals cracks**
- **Modular design is usually cheaper since expensive, mostly useless, cover is eliminated**

# Improved Design

- **Replace monolithic landfill type covers with modular designs**
- **Conceptually cover begins at top of buried structure, NOT land surface**
- **Clay layers, geomembranes, capillary barriers go as close to structure as possible (blanket the structure not the site)**
- **Vault width  $<$  (distance to boundary/10) to ensure proper mixing**
- **French drains to infiltrate water between vaults**
- **Modular design means less surface runoff to cause erosion**
- **Important barriers further beneath land surface – more robust**
- **Compatible with new buildings/ parking lots, etc above buried structure(s)**
- **Generally  $>10X$  lowering of dose while lowering costs and improving reliability**



expensive,  
unreliable  
high risk



lower cost  
reliable  
lower risk  
(vaults should also  
be smaller if possible)

# Other Important PA Issues

- **Probabilistic analysis: Peak of the mean analysis has methodological problems that cause systematic under estimates of risk**
- **Transients**
  - In nature transient events almost always cause peaks
  - in PA we mostly scale up steady state processes and ignore transients
  - more or earlier seepage is not always conservative
  - e.g., tank failure; leaky dam
  - storage by a barrier followed by failure of the barrier is critical (e.g., aging of iron corrosion products ( $K_d$  declines with time  $\rightarrow$  storage followed by release))
- **Management of preferential flow paths and stagnant regions within structures over time – backup drains**
- **Avoidance of “infallible barrier” proofs**
  - nearly impossible to prove
  - decrease public confidence
- **Managing how materials property changes over time interact with waste isolation performance**



# Conclusion

- **Traditional covers and designs are poor ideas that belong with landfills not buried structures**
- **Better engineering design is the low hanging fruit**
  - available today
  - lowers cost
  - improves performance
  - often counterintuitive
- **PA concepts have not filtered back to design**
- **PA analysts spend too much time analyzing poor designs and too little looking at new concepts**

# BACKUP SLIDES

# Ideal Design

- **Low cost**
- **Robust relative to materials degradation**
- **Does not unduly limit future land use**
- **Predictable performance bounds**
- **Low peak dose for all significant transport pathways**
- **Resistant to intrusion**
- **Avoids peaks or spikes in release rate**
- **Provides reliable mixing for any released contaminants**
- **Wherever practicable, delays release sufficiently long for maximization of decay**

# Why?

## Standard Cover

- barriers close to surface decreases reliability and longevity
- runoff causes erosion requiring expensive erosion barriers
- improper consideration of mixing
- leakage not low enough to reduce release

## Modular Buried Cover

- structural support for cover layers
- deeper burial of barriers increases longevity and reliability
- adjacent use of brittle and plastic barriers is optimal for reliability and low seepage
- mixing part of design ( $x < L/10$ )
- lower leakage, lower cost

The error function for a step function release for dispersion only (appropriate for transverse mixing of plumes) is:

$$\frac{C}{C_o} = \frac{1}{2} \operatorname{Erfc}(x/\sqrt{4Dt})$$

¶

Solving for  $C/C_o = 0.25$ , 25% mixing of plumes gives:

$$\frac{x}{\sqrt{4Dt}} = 0.477$$

Where  $x$  is the size of the facility and  $D$  is the dispersion coefficient. The transverse dispersivity of a groundwater plume is approximately 0.01\* scale, where the scale is the distance to the compliance boundary ( $L$ ).

$$\alpha = 0.01L$$

The ground water travel time to the boundary is:

$$t = L/v$$

The dispersion coefficient ( $D$ ) is given as:

$$D = \alpha v = 0.01Lv$$

Substitution gives:

$$\frac{x}{\sqrt{4Dt}} = 0.477 = \frac{x}{\sqrt{0.04L^2}}$$

$$x = 0.0954L \cong \frac{L}{10}$$



This gives the design constraint that the largest region of impermeable surface transverse the direction of groundwater flow should, as a criterion of engineering design, be less than 1/10 of the distance to the compliance boundary. ¶

$$x < L/10 ¶$$

Elimination of the concept of the landfill cover and replacing it with the above scaling relationship with mini-covers that begin at the top of the decommissioned structure rather than at the surface of the earth will lower costs and improve performance and reliability. ¶